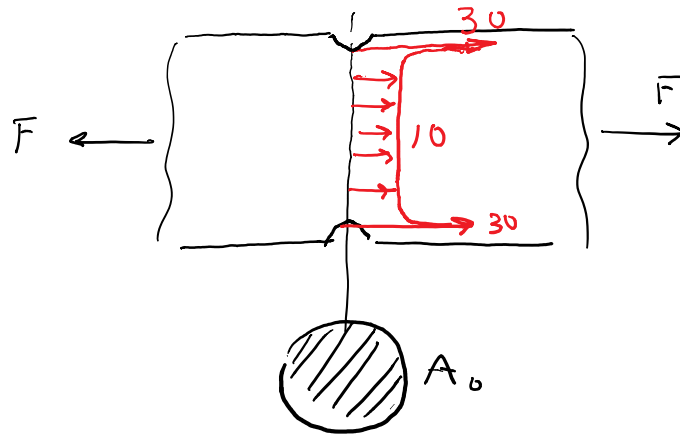


1/12/22

#1)



$$\sigma_{nom} = \frac{F}{A_0}$$

$$\sigma_{nom} = 10 \text{ ksi}$$

$$S_y = 60 \text{ ksi}$$

$K_t \equiv$ theoretical stress concentration factor

$$K_t = 3$$

$\sigma_{act} =$ Actual stress

$$\sigma_{theoretical} = K_t \sigma_{nom} = 3(10) = 30 \text{ ksi}$$

Answers:

- (F)

- (T)

- (F)

$$n = \text{Factor of Safety} = \frac{S_y}{\sigma_{nom}} = \frac{60}{10} = 6$$

For static loading & ductile behavior

We ignore K_t

#2)

$$\sigma_{nom} = 30$$

$$\sigma_{theoretical} = 90$$

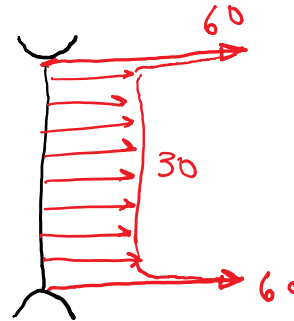
$$K_t = 3$$

$$S_y = 60$$

$$\sigma_{act} = 60 \text{ ksi}$$

- (F)

- (T)



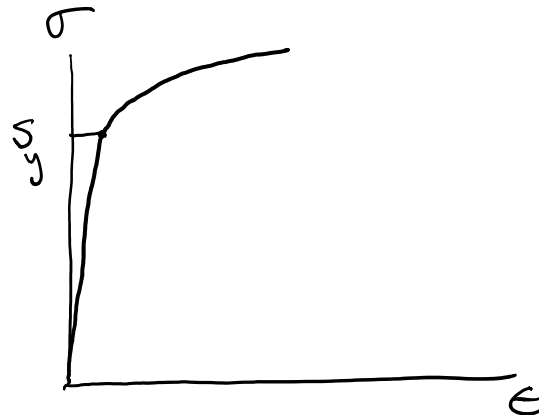
$$n = \frac{S_y}{\sigma_{nom}} = \frac{60}{30} = 2$$

- (F)

#3)

a. (F)

b. (T)



#4)

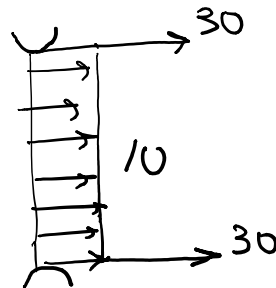
True stress

I) yielding is the onset of plastic deformation

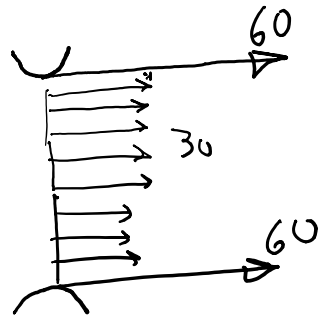
II) After yielding the material stretches a lot with little added force

#5)

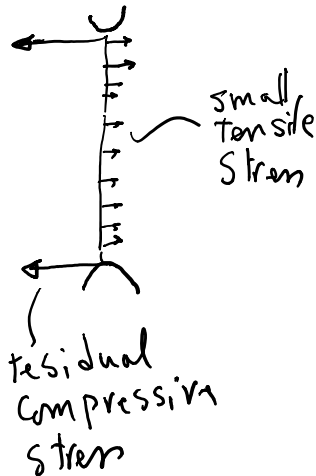
Ⓒ No stresses remain



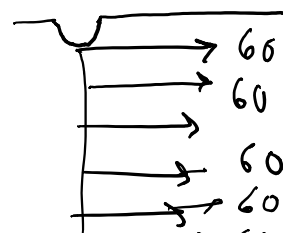
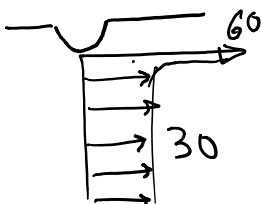
#6)

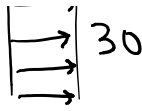


before load is removed

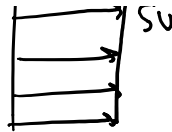


Ⓐ Compressive

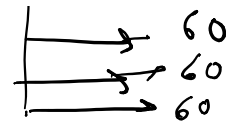




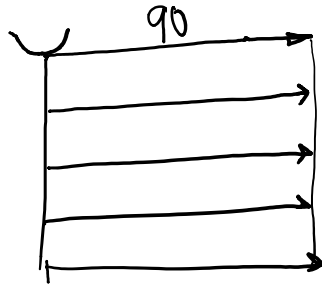
Start
No measurable
deformation



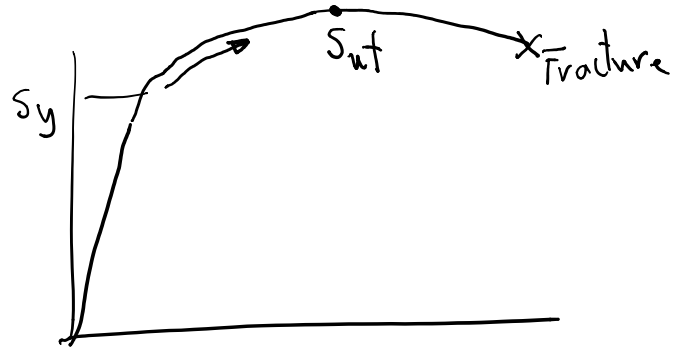
Add to stress
(force)
So $\sigma_{nom} = 50$



Add to stress
(force)
Start of measurable det.
So $\sigma_{nom} = 60$



Add to stress
So $\sigma_{nom} = 90 \text{ ksi}$



- A part fails only when bulk yielding occurs (measurable, objectionable deformation)

#7)



#8)



#9)



#10)

⊙ $\sigma_v \equiv \text{Von Mises stress}$

#11) Pure shear

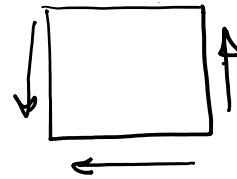


#11) Pure shear

$$\sigma_V = \sqrt{\sigma^2 + 3\tau^2}$$

$$\sigma = 0$$

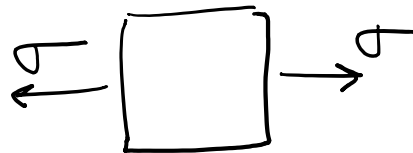
$$\sigma_V = \sqrt{3} \tau$$



#12

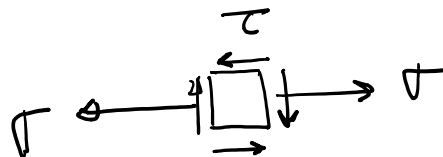
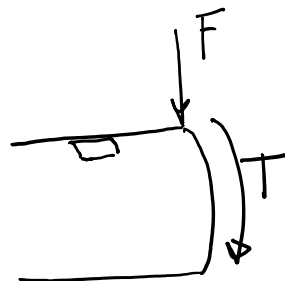
$$\sigma_V = \sqrt{\sigma^2}$$

$$\tau = 0$$



$$\sigma_V = \sigma$$

#13)



$$\sigma_V = \sqrt{\sigma^2 + 3\tau^2}$$

#14)

$$\textcircled{c} \quad \sigma_V = \sqrt{3} \tau$$

#15)

True

#16



#17)

(F)

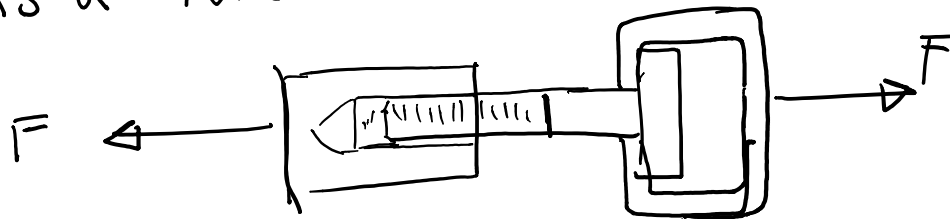
(T)

(F)

#18) Repeated yielding at areas of high stress concentration makes the material brittle and eventually a crack forms & propagates with each load application

Fastener Applications

As a Tension bar



Q

Given: 1.5 in UNC bolt

Grade 5

$F = 15000$ lbs

(Static loading)

Find: Factor of safety guarding against exceeding Proof Strength

(usually $K_t = 3.8$ but we will not need it)

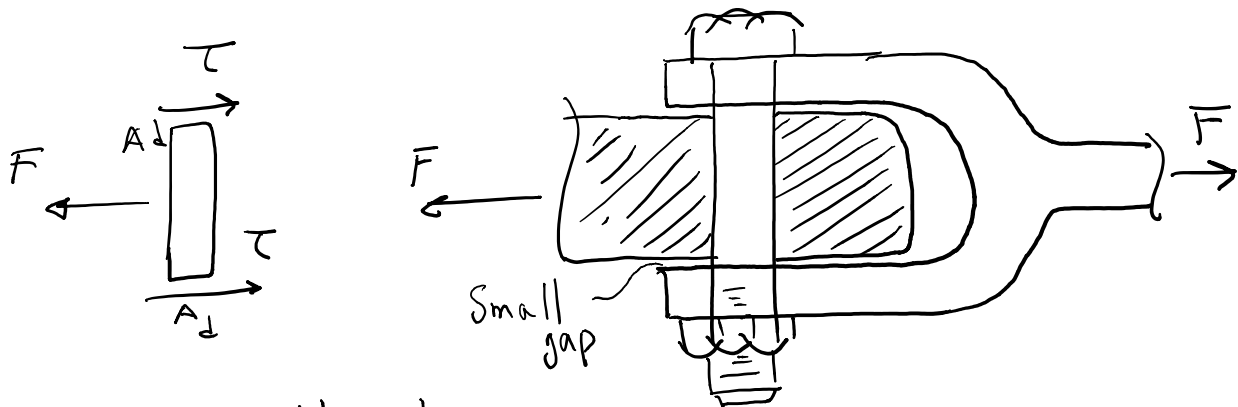
$$\tau = \frac{F}{A} = \frac{15000}{\dots} = 10676.1 \text{ Psi}$$

$$U_n \bar{A}_t \quad 1.405$$

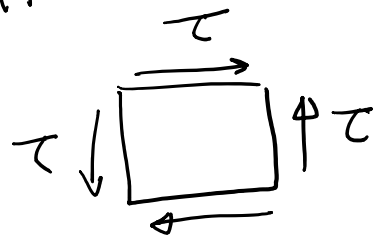
$$S_p = 74000 \text{ PSI}$$

$$n = \frac{S_p}{\sigma} = \frac{74000}{10676} = 6.9$$

Application \equiv As a Shear Pin



Double Shear
Tongue & clevis joint



Pure Shear stress element

Given:

1 in UNC

$F = 10,000 \text{ lbs}$

Grade 2

Find: Factor of Safety guarding against (bulk) yielding of the shank cross section

n..

$$\tau_{nom} = \frac{\bar{F}}{2 A_d} = \frac{10\,000}{2(0.785)} = 6366.2 \text{ psi}$$

where

$$A_d = \frac{\pi d^2}{4} = 0.785$$

Given $S_y = 36 \text{ Ksi}$

$$n = \frac{S_y}{\sigma_v} = \frac{S_y}{\sqrt{3} \tau} = \frac{0.58 S_y}{\tau}$$

$$n = \frac{0.58 (36000)}{6366.2} \approx 3.3 \leftarrow$$

Application \equiv Fastener used to clamp plates

Given 1.5 in UNC

$$K_b = 0.8 K_m$$

$$F_i \equiv 75\% \text{ Proof strength}$$

$$F_e = 20\,000 \text{ lbs}$$

Find: Load factor guarding against bolt exceeding its proof load

$$n_L = \text{load factor} = \frac{F_e^*}{F_e}$$

F_e^* = external load leading to bolt exceeding its proof strength

\bar{F}_e = Actual external load

Joint index

$$C = \frac{K_b}{K_b + K_m} \Rightarrow C = 0.44$$

Initial tension

$$F_i = 0.75 F_p = 0.75 (46365) = 34776 \text{ Psi}$$

where

$$F_p = A_t S_p = 1.405 (33000) = \underbrace{46365}_{\text{Proof load}}$$

where

$$A_t = 1.405$$

$$S_p = 33000 \text{ Psi}$$

Bolt load

$$\bar{F}_b = F_i + C F_e$$

$$\bar{F}_b = 34.77 + 0.44 (20) \text{ Kips}$$

$$\bar{F}_b = 43.57 \text{ Kips}$$

External load F_e^* causing the bolt to exceed its proof strength is

$$F_b = F_i + C F_e$$

$$\text{Sub. } F_b = F_p = 46.365 \text{ kips}$$

$$46.365 = 34.77 + 0.44 F_e^*$$

$$\Rightarrow F_e^* = 26.35 \text{ kips}$$

Load factor

$$M_L = \frac{F_e^*}{F_e} = \frac{26.35}{20} = 1.32 \leftarrow$$
