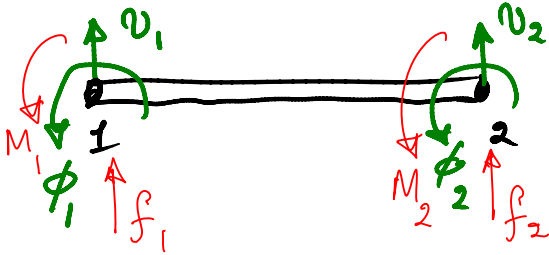


CHAP. 4 (BEAM ELEMENTS)

- BEAM ALLOWS FOR FLEXURE (BENDING).
- SIMPLEST MEMBER OF BEAM HAS 4 D.O.F.'S



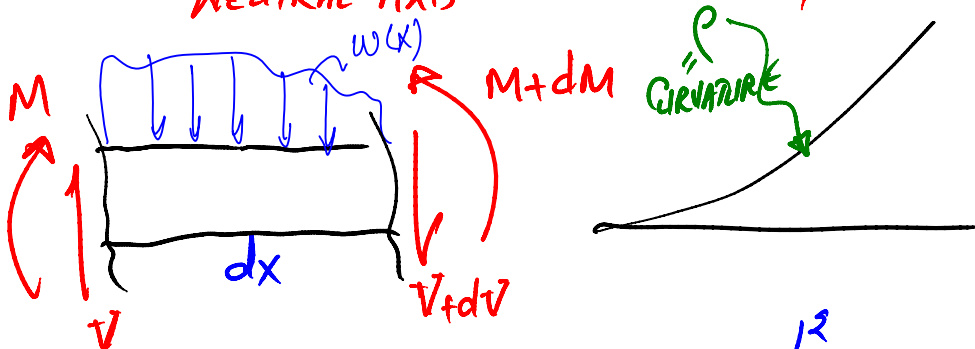
BEAM FORMULATIONS:

1) EULER (BERNOULI) BEAM - LONG, SLENDER

- TRANSVERSE SHEAR IS IGNORED.
- PLANE X-SECTIONS REMAIN PLANE AND PERP. TO NEUTRAL AXIS

2) TIMOSHENKO (MINDLIN) BEAM - SHORT, THICK

TRANSV. SHEAR IS IMPORTANT, PLANE X-SECTIONS REMAIN PLANE, NOT NECESSARILY PERP. TO NEUTRAL AXIS



$$V = \frac{dM}{dx} \quad \frac{1}{\rho} = \frac{M}{EI} \quad \frac{1}{\rho} = \frac{d^2 w}{dx^2}$$

CURVATURE

$$\frac{M}{EI} = \frac{d^2 w}{dx^2}$$

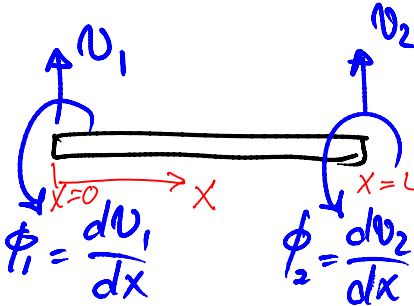
⇒ IN TERMS OF DISTRIBUTED LOAD:

$$\frac{d^2}{dx^2} \left(EI \frac{d^2 w}{dx^2} \right) = -w(x)$$

$$\frac{d^2}{dx^2} \left(EI \frac{d^2 v}{dx^2} \right) = -w(x)$$

ASSUMING E, I ARE CONSTANT FOR EACH ELEM.

$$EI \frac{d^4 v}{dx^4} = -w(x)$$



ASSUMED DISPLACEMENT:

$$v(x) = a_1 + a_2 x + a_3 x^2 + a_4 x^3$$

MIN. POLYNOMIAL ORDER

APPLYING B.C.'S \Rightarrow $\begin{cases} x=0 & v = v_1 \\ & \phi = \frac{dv_1}{dx} \end{cases}$

$\begin{cases} x=L & v = v_2 \\ & \phi = \frac{dv_2}{dx} \end{cases}$

$$\frac{dv}{dx} = a_2 + 2a_3 x + 3a_4 x^2$$

SOLVE TO OBTAIN:

$$v(x) = [N_1 \ N_2 \ N_3 \ N_4] \begin{Bmatrix} v_1 \\ \phi_1 \\ v_2 \\ \phi_2 \end{Bmatrix}$$

$$v(x) = [N] \{d\}$$

$$N_1 = \frac{1}{L^3} (2x^3 - 3x^2 L + L^3)$$

$$N_2 = \frac{1}{L^3} (xL - 2x^2 L^2 + xL^3)$$

$$N_3 = \frac{1}{L^3} (12x - 6L)$$

$$N_4 = \dots$$

FORM STRAIN ENERGY: $U = \frac{1}{2} \int_{Vol} \{\sigma\}^T \{\epsilon\} dVol.$

$$\epsilon_{BEAM} = -y \frac{d^2 v}{dx^2}$$

$$\{\sigma\} = [E] \{\epsilon\}$$

$$\{\epsilon\} = [B] \{d\} (-y)$$

STRAIN-DISP. MATRIX

$$v = [N] \{d\} \Rightarrow \frac{dv}{dx} = \frac{d[N]}{dx} \{d\} \Rightarrow \frac{d^2 v}{dx^2} = \frac{d^2 [N]}{dx^2} \{d\}$$

$$\frac{d^2 [N]}{dx^2} = \left[\frac{1}{L} (12x - 6L) \quad \dots \quad \dots \quad \dots \right]$$

$$\{ \epsilon \} = -\gamma \left(\frac{d^2 w}{dx^2} \right) \{ d \} \quad \sigma = E \{ \epsilon \} = E \left(-\gamma \left(\frac{d^2 w}{dx^2} \right) \{ d \} \right)$$

$$\{ \sigma \}^T = E \{ d \}^T [B]^T (-\gamma) [B]$$

$$U = \frac{1}{2} \int_{Vol} \{ \sigma \}^T [B]^T \{ \epsilon \} dVol.$$

$$U = \frac{1}{2} \int_{Vol} \underbrace{(-\gamma E \{ d \}^T [B]^T)}_{\{ \sigma \}^T} \cdot \underbrace{(-\gamma [B] \{ d \})}_{\{ \epsilon \}} dVol.$$

$$U = \frac{1}{2} \int_A \gamma^2 dA \cdot \int_{Length} E \{ d \}^T [B]^T [B] \{ d \} dx$$

$I \equiv$ AREA MOMENT OF INERTIA L

$$U = \frac{1}{2} \{ d \}^T \left(EI \int_0^L [B]^T [B] dx \right) \{ d \}$$

COMPARE:

$$U = \frac{1}{2} \{ x \}^T [K] \{ x \}$$

$$[K]_{BEAM} = EI \int_0^L \underbrace{[B]^T}_{4 \times 1} \underbrace{[B]}_{1 \times 4} dx$$

$$[B] = \begin{bmatrix} 12x-6L & 6xL-4L^2 & -12x+6L & 6xL-2L^2 \end{bmatrix}$$

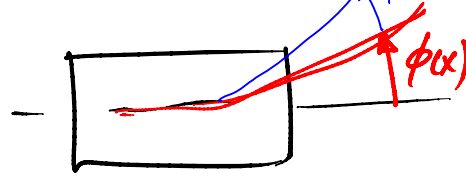
$$[K] = \frac{EI}{L^3} \begin{bmatrix} 12 & 6L & -12 & 6L \\ 6L & 4L^2 & -6L & 2L^2 \\ -12 & -6L & 12 & -6L \end{bmatrix}$$

... W... LEAD

$$\begin{bmatrix} 6L & 2L^2 & -6L & 4L^2 \end{bmatrix}$$

(TIMOSHENKO) SHEAR COMPONENT RESULT

TIMOSHENKO (MINDLIN)



$$\frac{dV}{dx} = \phi(x) + \beta(x)$$

$$M(x) = EI \frac{d\phi(x)}{dx}$$

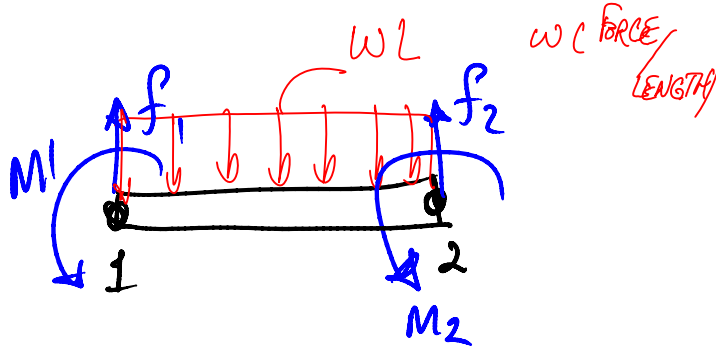
$$V(x) = k_s \cdot A \cdot G \cdot \beta(x)$$

\downarrow SHEAR STRESS
 \downarrow AREA
 \downarrow SHEAR MODULUS
 TIMOSHENKO COMPONENT

$$[k]_{\text{TIMOSHENKO}} = \frac{EI}{L^3(1+\varphi)}$$

$$\begin{bmatrix} 12 & 6L & -12 & 6L \\ 6L & (4+\varphi)L^2 & -6L & (2-\varphi)L^2 \\ -12 & -6L & 12 & -6L \\ 6L & (2-\varphi)L^2 & -6L & (4+\varphi)L^2 \end{bmatrix}$$

$$\varphi = \frac{12EI}{k_s A G L^2}$$



DISTRIBUTED LOAD

ALL FORCE TERMS:

$$\{ \hat{f} \} = - \int_{\text{Vol.}} \hat{x} [N] \{ \hat{d} \} d\text{vol} - \int_{\text{SUR.}} \hat{T} [N] \{ \hat{d} \} da - \{ \hat{P} \}$$

\downarrow Vol. FORCE
 \downarrow SUR. TRACTION
 \downarrow POINT LOAD

$$\text{DIST. LOAD} = \int_0^L w(x) [N]^T dx$$

WORK-EQUIVALENT METHOD:

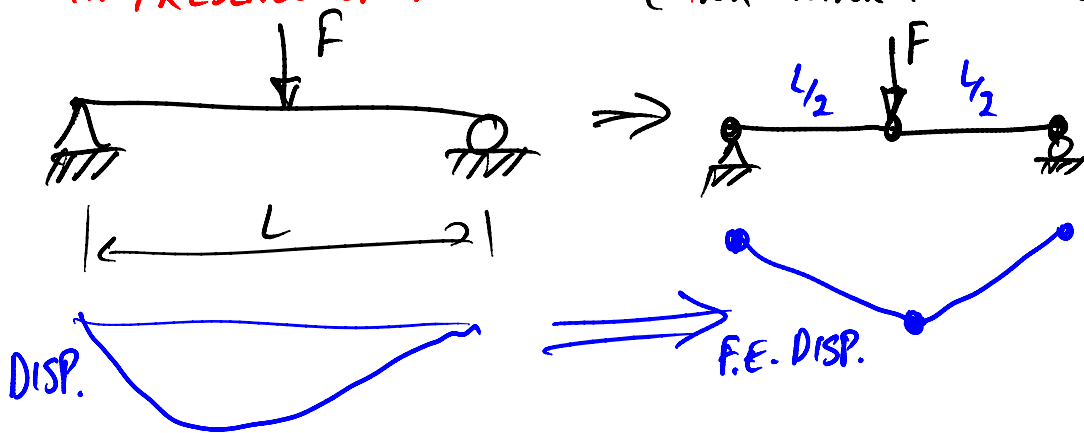
WORK OF DISTRIBUTED LOAD IS EQUIV. TO WORK DONE BY FORCES AND MOMENTS APPROXIMATED AS F.E. LOADS.

$$W = m_1 \hat{\phi}_1 + m_2 \hat{\phi}_2 + f_1 \hat{d}_1 + f_2 \hat{d}_2$$

$$W = \int_0^L w(x) v(x) dx$$

↓
POLYNOMIAL ORDER FOR DISTRIBUTED LOAD IS, AT LEAST ONE ORDER HIGHER THAN DISP. FIELD.

THEREFORE, USE TWICE AS MANY ELEM. DISTRIBUTIONS IN PRESENCE OF DIST. LOAD. (FOR UNIFORM DIST. LOAD).



NOTE: F.E. MODEL DOES NOT REPRESENT DEFORMED SHAPE WITH MINIMUM ELEMENT DISCRETIZATION.

USE 3 ELEMENTS PER SPAN TO OBTAIN "SMOOTH" DISP. FIELD.

