

Population: 9000 Peak Q = (105 gpcd)(9000)(1.8) = **1.70 MGD**

Retention Pond

↑ Typical, but any value near this OK

↑ Recommended 180% peak day (yearly) Other peaking factors for peak day (not hour) are OK.

Can design for either peak or average flow. Assignment says 4-h AVERAGE flow, but could be more conservative & size for peak day.

Going metric here: $(1.70 \times 10^6 \frac{\text{gal}}{\text{d}})(3.78 \times 10^{-3} \frac{\text{m}^3}{\text{gal}}) = 6,430 \text{ m}^3/\text{d}$ PEAK FLOW

$A_s = \frac{Qtd}{d} = \frac{(6,430 \text{ m}^3/\text{d})(4 \text{ h})(1/24 \frac{\text{d}}{\text{h}})}{2 \text{ m}} = 540 \text{ m}^2$ PEAK FLOW

300 m² AVE. FLOW

I° CLARIFIER:

$V_s = 30 \text{ m/d} \Rightarrow A_s = \frac{Q}{V_s} = \frac{6,430 \text{ m}^3/\text{d}}{30 \text{ m/d}} = 214 \text{ m}^2$

Given { L/w Ratio from 3:1 → 5:1
W either 4m OR 5m

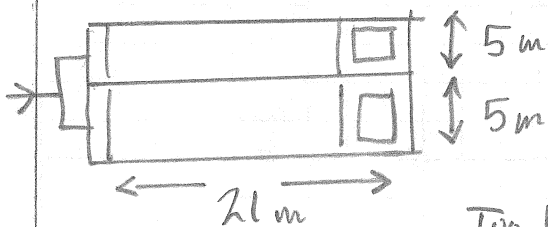
Try smaller 4 m: $L = \frac{A_s}{w} = \frac{214 \text{ m}^2}{4} = 54 \text{ m}$ $\frac{L}{w} = \frac{54}{4} = 13$ WAY TOO LONG

IF we use 2 basins, each 4 m, still $L/W = \frac{13}{2} = 6.5$, too long

SO, use 2 basins, each 5 m: W = 10 m TOTAL

The $L = \frac{A_s}{w} = \frac{214 \text{ m}^2}{10} = 21 \text{ m}$ $\frac{L}{w} = \frac{21}{5} = 4.2$ OK

LAYOUT & WEIRS



MAX. WEIR LOADING

$250 \text{ m}^3/\text{m}\cdot\text{d}$

MIN. WEIR LENGTH = $\frac{6,430 \text{ m}^3/\text{d}}{250 \text{ m}^3/\text{m}\cdot\text{d}}$

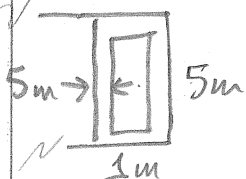
← EACH BASIN IS 5m

= **26 m**

Too long for a single weir across 10m Total W.

ANY geometry of weirs that gets you to > 26 m is OK.

I came up with simple & conservative layout.



This gets you $L_T = (5 + 5 + 5 + 1 + 1) \text{ m} = 17 \text{ m}$

X 2 TANKS

34 m

LOADS of Effluent BOD & TSS:

Average Loads for 35% removal of BOD & 50% for Solids

$$\text{BOD}_5: (200 \text{ mg/L})(1 - 0.35) = \boxed{130 \text{ mg/L}} \text{ conc.}$$

$$(130 \text{ mg/L})(1.70 \text{ MGD})(8.34) = \boxed{1840 \text{ lb/d BOD}} \text{ LOAD}$$

$$\text{TSS: } (200 \text{ mg/L})(1 - 0.5) = \boxed{100 \text{ mg/L}} \text{ conc.}$$

$$(100 \text{ mg/L})(1.70 \text{ MGD})(8.34) = \boxed{1,420 \text{ lb/d SOLIDS-DRY}}$$

SLUDGE: 1% solids, so wet weight is 100 times dry weight

$$(1,420 \text{ lb/d})(100) = \boxed{142,000 \text{ lb/d SLUDGE-WET}}$$

PEAK LOADS Assignment did not ask for peak loads but you can calculate some estimates using the so-called "generic" (regression) equations in lecture notes.

PEAK DAY

$$B_d = B \left(\frac{4.08}{B^{0.0732}} \right) = (1840) \left(\frac{4.08}{1840^{0.0732}} \right) = 1840 \times 2.35$$

$\left. \begin{matrix} B \\ S \end{matrix} \right\} = \text{ann. averages, lb/d}$

$$= \boxed{4,330 \text{ lb/d}}$$

$$S_d = S \left(\frac{5.98}{S^{0.0716}} \right) = (1420) \left(\frac{5.98}{1420^{0.0716}} \right) = 1,420 \times 3.56$$

$$= \boxed{5,050 \text{ lb/d}}$$

For comparison to 180% used above, check Qd peaking factor by this method:

$$Q_d = Q \left(\frac{1.96}{Q^{0.0369}} \right) = 1.70 \left(\frac{1.96}{1.70^{0.0369}} \right) = (1.70 \text{ MGD})(1.92) = \boxed{3.3 \text{ MGD}}$$

QUITE CLOSE: 190% vs 180%